

**EFFECTS OF EXCAVATION SEQUENCE AND HEADING  
DISTANCE ON SETTLEMENT IN NEW AUSTRIAN  
TUNNELING METHOD**

**HOUMAN SOHAEI**

**UNIVERSITI TEKNOLOGI MALAYSIA**

Effects of Excavation Sequence and Heading Distance on Settlement in New  
Austrian Tunneling Method

HOUMAN SOHAEI

A dissertation submitted in partial fulfilment of the  
requirements for the award of the degree of  
Master of Engineering (Civil-Geotechnics)

Faculty of Civil Engineering  
Universiti Teknologi Malaysia

OCTOBER 2012

Dedicated with much love and affection to my beloved mother and father,  
and all my family who always supported me

## **ACKNOWLEDGEMENTS**

In preparing this dissertation, I was in contact with many people, researchers, academicians, and practitioners. They have contributed towards my understanding and thoughts. In particular, I wish to express my sincere appreciation to my dissertation supervisor, Professor Dr. Aminaton bt. Marto, for encouragement, guidance, critics and friendship. Without her continued support and interest, this dissertation would not have been the same as presented here. I would like to thanks the Universiti Teknologi Malaysia (UTM) for providing facilities in the Faculty of Civil Engineering (FKA) during my study. The help from Tunnel Rod Company in providing data for the case study is highly appreciated. Thanks are also due to all technicians of Computer Laboratory, FKA, UTM for their assistance in the use of computer facilities.

My fellow postgraduate students should also be recognized for their supports in particular Mr. Mohsen Hajihassani and Mr. Eshagh Namazi. My sincere appreciation also extends to all colleagues and researchers in the Soft Soil Engineering Research Group, and others who have provided assistance at various occasions. Their views and tips are useful indeed. Unfortunately, it is not possible to list all of them in this limited space.

Last but not least, I would like to acknowledge the emotional and financial supports given by my family members. Their continued support and encouragement enlighten my day and give motivation to complete my study in due time.

## ABSTRACT

The shallow underground excavation may leads to ground movements and surface settlement which may cause damage to structures. Several tunnel excavation methods had been developed during the last decades to minimize the effects of the tunnel construction on the surface settlement. The Karaj Metro tunnel (KMT) had been constructed in accordance with the principles of the New Austrian Tunneling Method (NATM). This method had been used widely to construct large diameter tunnels mainly due to its flexibility to adapt different ground conditions. Tunnel designs by NATM are generally based on empirical and numerical methods and construction process may be changed according to the observed response of the ground. Induced displacements are empirically controlled by adjusting the excavation rate, distance between tunnel face and support, partial heading excavation and closure of invert. This research is aimed at determining the effects of the excavation sequence and heading distance on the surface and subsurface settlement by carrying out two and three-dimensional Finite Element Modelling (FEM). Initially, the FEM is carried out to simulate step by step excavation sequence of KMT which had been constructed in soft soils by NATM method. The settlements obtained from monitoring of KMT had been used to validate the modelling work. The results show that the settlement varies with different excavation sequence and heading distance in NATM. The Side Galleries (SG) excavation model produced the lowest transverse and longitudinal surface settlements compared to KMT excavation model and other excavation sequences. The tunnel heading distance had more effect on both the transverse and longitudinal settlements for the KMT excavation model compared to SG model. Hence, the SG excavation model with heading distance of 2 m is recommended in the construction of KMT using NATM based on the minimum settlement occurring during excavation.

## ABSTRAK

Kerja-kerja pengorekan cetek bawah tanah boleh menyebabkan pergerakan tanah dan enapan permukaan, dan seterusnya boleh mengakibatkan kerosakan kepada struktur. Beberapa kaedah pengorekan terowong telah dihasilkan sepanjang beberapa dekad yang lalu untuk mengurangkan kesan kerja pembinaan terowong terhadap enapan permukaan tanah. Terowong "Karaj Metro" (KMT) telah dibina menurut prinsip "New Austrian Tunneling Method" (NATM). Kaedah pembinaan ini telah digunakan dengan meluas untuk membina terowong bergarispusat besar kerana kaedah ini sesuai digunakan untuk pelbagai keadaan tanah. Rekabentuk terowong yang dihasilkan menggunakan kaedah NATM adalah berdasarkan kepada kaedah-kaedah empirikal dan berangka, manakala proses pembinaan boleh diselaraskan menurut perubahan dan pergerakan tanah yang dipantau secara berterusan. Enapan yang teraruh oleh kerja pengorekan boleh dikawal secara empirikal dengan menyelaraskan kadar pengorekan, jarak antara permukaan terowong dan penyokongan, pengorekan separa permukaan terowong, serta pengecutan bumbung terowong. Penyelidikan ini bertujuan untuk menentukan kesan urutan pengorekan serta kesan jarak permukaan terowong terhadap enapan di permukaan dan sub-permukaan tanah dengan menghasilkan model unsur terhingga "Finite Element Modelling" (FEM) dalam dua dimensi dan tiga dimensi. Pada mulanya FEM diguna untuk mensimulasi urutan proses pengorekan secara langkah demi langkah bagi projek KMT yang telah dibina di dalam tanah lembut dengan kaedah NATM. Bacaan enapan tanah yang diperolehi daripada pengawasan proses pembinaan Terowong "Karaj Metro" telah digunakan untuk mengesahkan kerja-kerja pemodelan yang telah dilakukan. Keputusan yang diperolehi daripada kerja pemodelan tersebut menunjukkan bahawa nilai enapan tanah berubah dengan perubahan kepada urutan proses pengorekan dan jarak maju terowong di dalam kaedah NATM. Model pengorekan "Side Galleries" (SG) menghasilkan enapan-enapan melintang dan memanjang yang terendah dibandingkan dengan model pengorekan KMT dan urutan pengorekan yang lain. Jarak maju terowong memberi kesan yang lebih besar keatas enapan-enapan melintang dan memanjang bagi model pengorekan KMT dibandingkan dengan model SG. Justeru, model pengorekan SG dengan jarak pengorekan 2 m disyorkan untuk pembinaan KMT dengan menggunakan kaedah NATM berdasarkan kepada nilai enapan minimum yang berlaku semasa pengorekan.

## TABLE OF CONTENTS

CHAPTER	TITLE	PAGE
	<b>TITLE PAGE</b>	<b>i</b>
	<b>DECLARATION</b>	<b>ii</b>
	<b>DEDICATION</b>	<b>iii</b>
	<b>ACKNOWLEDGEMENTS</b>	<b>iv</b>
	<b>TABLE OF CONTENTS</b>	<b>vii</b>
	<b>LIST OF TABLES</b>	<b>xi</b>
	<b>LIST OF FIGURES</b>	<b>xii</b>
	<b>LIST OF SYMBOLS</b>	<b>xvi</b>
	<b>LIST OF APPENDICES</b>	<b>xviii</b>
<b>1</b>	<b>INTRODUCTION</b>	<b>1</b>
	1.1 Background of the Study	1
	1.2 Problem Statement	2
	1.3 Aim and Objectives of the Study	3
	1.4 Significance of the Study	4
	1.5 Scope and limitation of the Study	4
<b>2</b>	<b>LITERATURE REVIEW</b>	<b>5</b>
	2.1 Introduction	5
	2.2 Tunneling	5
	2.3 Tunnel and Excavation Method	6

2.3.1	Tunneling Based on Construction Technique	6
2.3.2	Tunneling Based from a Tunneling Perspective	7
2.3.3	Selecting Excavation Method	8
2.3.4	Soil Classifications in Tunneling View	8
2.4	New Austrian Tunneling Method	10
2.4.1	Introduction	10
2.4.2	Construction Method	10
2.4.3	NATM in Rock	12
2.4.4	NATM in Loose Ground	12
2.5	Multiface Excavation Design in NATM	14
2.6	Shotcrete Supporting Technique in Tunneling	15
2.7	Stresses Around the Tunnel	17
2.8	Settlement Due to Tunneling	19
2.8.1	Transverse Settlement	21
2.8.1.1	Volume Loss Method	22
2.8.1.2	Classical Method of the Settlement Analysis	25
2.8.2	Longitudinal Settlement	26
2.9	Ground Deformation During Tunneling Process	27
2.10	Effect of Excavation Sequence in NATM	28
2.11	Numerical Methods	30
2.11.1	Introduction	30
2.11.2	Finite Element Method	31
2.11.3	Previous Studies on FEM in Tunneling	32
2.12	Commercial Finite Element Software	36
2.12.1	Plaxis Software	36
2.12.2	Abaqus Software	38
<b>3</b>	<b>RESEARCH METHODOLOGY</b>	<b>40</b>
3.1	Introduction	40
3.2	Theory, Background and Literature Review	41
3.3	Data Collection	42
3.3.1	Soil Parameters	43
3.3.2	Laboratory Test Data	43
3.3.3	Settlement Data	44



3.4	2D and 3D Finite Element Analyses	44
3.5	Validation	46
<b>4</b>	<b>CASE STUDY</b>	<b>47</b>
4.1	Introduction	47
4.2	Geological Conditions	48
4.3	Sub-surface Geotechnical Conditions	48
4.3.1	Overburden Soil Layer	49
4.3.2	Coarse Soil Layer (L1)	49
4.3.3	Fine-Grained Soil Layers (L2)	49
4.3.3.1	Sub-layer 1 (L2-1)	50
4.3.3.2	Sub-layer 2 (L2-2)	50
4.4	Overall Soil Properties	50
4.5	Tunnel Geometry and Tunneling Function	51
4.6	Settlement Monitoring	54
<b>5</b>	<b>FINITE ELEMENT ANALYSIS</b>	<b>55</b>
5.1	Introduction	55
5.2	Plaxis 2D Finite Element Modeling	56
5.3	Abaqus 3D Finite Element Modeling	60
5.3.1	Model Geometry	60
5.3.2	Finite Element Mesh	62
5.3.3	Simulation of Karaj Metro Tunnel Construction	63
5.3.4	Soil Model	65
<b>6</b>	<b>RESULTS AND DISCUSSIONS</b>	<b>66</b>
6.1	Introduction	66
6.2	2D FE Results	66
6.2.1	Settlement of KMT	67
6.2.2	Effect of Excavation Sequence on Surface Settlement	68
6.2.3	Effect of Excavation Sequence on Subsurface Settlement	68
6.2.4	Stress Distribution	69
6.3	3D FE Modeling Results	70
6.4	Settlement Control with Side Galleries Method	73

6.4.1	Simulating the Side Galleries Method	74
6.4.2	Transverse Settlement	75
6.4.3	Longitudinal Settlement	77
6.4.4	Settlements at Different Monitoring Points for KMT and SG Methods	78
6.5	Optimal Face Distance between Top Heading and Bench	80
6.6	Effects of Different Heading Distance on Settlement	83
6.7	Movement-Lining Interaction Assessment	84
<b>7</b>	<b>CONCLUSIONS AND RECOMMENDATIONS</b>	<b>89</b>
7.1	Introduction	89
7.2	Conclusions	89
7.3	Recommendations for Further Research	91
	<b>REFERENCES</b>	<b>92</b>
	Appendices A - B	95 - 100

## LIST OF TABLES

TABLE NO.	TITLE	PAGE
2.1	Tunneling ground classification system (Heuer, 1987)	9
2.2	Coefficient of influence point (k) (Toan, 2006)	24
2.3	Volume loss parameter $V_L$ (Toan , 2006)	24
3.1	Subsurface soil layers in Phase I of Karaj Metro Tunnel, Iran	43
4.1	Geotechnical properties of soil layers in Phase I of Karaj Metro Tunnel (Tunnel Rod, 2011)	51
5.1	Geotechnical properties of soil layers at Karaj Metro Tunnel	56
5.2	Shotcrete materials of Karaj Metro Tunnel	56

## LIST OF FIGURES

FIGURE NO.	TITLE	PAGE
2.1	Cut and cover tunneling method (Megaw and Bartlett, 1981)	7
2.2	Typical tunnel face excavated by NATM method (Powell, 2001)	13
2.3	Several sections of face excavation in NATM Tunneling (Kolymbas, 2005)	15
2.4	Applying layer of shotcrete on reinforcement mesh (Marcher and Jiricny, 2004)	16
2.5	Diversion of flow lines around a tunnel under uniaxial stress (Morrison, 1976)	18
2.6	Stress distribution around a tunnel under uniaxial stress (Budavari, 1983)	18
2.7	Distribution of tangential stresses on the boundary of a tunnel (Budavari, 1983)	19
2.8	Tunnel settlement trough (Lee <i>et al.</i> , 2001)	21
2.9	Surface settlement across the tunnel axis (Peck <i>et al.</i> , 1969)	22
2.10	The settlement trough and volume loss $V_L$ (Moller, 2006)	23
2.11	Longitudinal settlements due to tunneling (Woodman, 1982)	27
2.12	Stress reduction form adopts the ground deformation curve (Moller, 2006)	28
2.13	Various excavation models used for Heathrow Tunnel in London (Bowers, 1997)	29

2.14	Excavation method of single sidewall drifts (Bowers, 1997)	29
2.15	Predicted transverse surface settlement profiles by FLAC and Abaqus (Karakus and Fowell, 2003)	32
2.16	Cross-section and excavation sequence in ring-cut method (Yoo, 2009)	33
2.17	Cross-section and excavation sequence in sidewall drift method (Yoo, 2009)	34
2.18	Settlement occur from single cut (Möller <i>et al.</i> , 2008)	34
2.19	Ground Profile of London tunnel (Ng <i>et al.</i> , (2004))	35
2.20	Longitudinal Settlement at sections 1-1, 2-2, and 3-3 for distance of 3.5D between twin tunnels (Ng. <i>et a.l</i> , 2004)	36
2.21	Nodes and stresses points (Brinkgreve, 2002)	37
3.1	Study flow chart	41
3.2	Various excavation sections of the tunnel face	45
4.1	Location of Karaj City, North of Iran	47
4.2	Geotechnical section in tunnel cross section area (Sahel Consultant Engineers, 2005)	48
4.3	Location of subway stations in Line 2 of KMT project	52
4.4	Tunnel profile and excavation sequence of KMT (Tunnel Rod, 2012)	53
4.5	Excavation dimensions	53
4.6	Cross section of the tunnel and the locations of settlement markers	54
5.1	Geometry dimensions of tunnel profile	57
5.2	Geotechnical dimension and boundary conditions	58
5.3	Plaxis meshing	59
5.4	Plaxis output	59
5.5	Geometry of the model in FE analysis	61
5.6	Number of mesh and element	63

5.7	KMT excavation process	64
6.1	Transverse settlement trough from field monitoring data and FE analysis of Karaj Metro Tunnel	67
6.2	Transverse settlement troughs due to various excavation sequences	68
6.3	Sub surface settlement at tunnel centreline due to various excavation sequences	69
6.4	Stress distributions in different excavation sequence in existing Karaj Metro Tunnel (Section A of Figure 3.3)	70
6.5	Stress distributions in different excavation sequence of side galleries excavation method (Section E of Figure 3.3)	70
6.6	Ground movements around the tunnel obtained by FE analysis based on KMT excavation model	71
6.7	Comparison between FEM cumulative surface settlements at monitoring Point A for KMT excavation model with measured data	72
6.8	Comparison between longitudinal surface settlements obtained by FEM and measured data for KMT excavation model	73
6.9	Side Galleries excavation method (Kolymbas, 2005)	73
6.10	Side Galleries excavation process	75
6.11	Ground movements around the tunnel excavated by Side Galleries method obtained by FE analysis	76
6.12	Comparison on the transverse surface settlement obtained from Karaj Metro Tunnel excavation model and SG excavation model	77
6.13	Comparison of longitudinal surface settlement for measured and FE predicted KMT model, and FE predicted SG model	78
6.14	Monitoring point on the tunnel axis at ground surface	79
6.15	Cumulative surface settlement on monitoring Point B for KMT and SG models during tunneling operation	80
6.16	Cumulative surface settlement on monitoring Point C for KMT excavation model during tunneling operation	81
6.17	Cumulative surface settlement on monitoring Point C for suggested Side Galleries excavation model	82

6.18	Transverse and longitudinal settlements at different excavation advancement length	84
6.18	Monitoring point on tunnel axis and lining	85
6.19	Vertical Earth pressure at initial soil condition	86
6.20	Vertical earth pressure on the tunnel crown of KMT model at different steps	86
6.21	Vertical earth pressure on the tunnel crown of SG model at different steps	87

## LIST OF SYMBOLS

$\gamma$	-	Unit weight
$E$	-	Young's modulus
$\nu$	-	Poisson's ratio
$\phi$	-	Internal friction angle
$c$	-	Cohesion
$k$	-	Permeability
$K_0$	-	Coefficient of lateral earth pressure at rest
$N$	-	Standard Penetration Test (SPT) Value
$A$	-	Excavation area
$Z$	-	Center point of excavation depth
$V_L$	-	Percentage of volume loss
$L_{inf}$	-	Location of inflection point
$S_v$	-	Vertical settlement on X-direction
$V_s$	-	Volume of the curve area
$\sigma_z$	-	Soil stress at the centre of excavation
$K_r$	-	Coefficient of earth pressure at rest of cohesive soil
$S_{(y,z)}$	-	Vertical displacement



$H_{(y,z)}$	-	Horizontal displacement
$i$	-	Influence point
$\beta$	-	Reduction factor
$P_r$	-	Ground pressure due to stress reduction
$P_0$	-	Initial ground pressure
$r$	-	Tunnel radius

**LIST OF APPENDICES**

<b>APPENDIX</b>	<b>TITLE</b>	<b>PAGE</b>
2.1	Phases in excavation process of KMT excavation	95
2.2	Phases in excavation process of Side Galleries excavation model	98

## **CHAPTER 1**

### **INTRODUCTION**

#### **1.1 Background of the Study**

Underground construction has become an important factor to reduce the congestion at the ground surface due to swift growth at the major cities. Tunnels are an important part of underground construction and the demand for tunnel transportation and safety excavation had been significantly increased for the development of rail or road tunnel systems in recent years. Hence, tunnels are increasingly used in the urban areas due to the growth of population.

Since tunnels have been constructed over hundred years, the design has improved gradually based on analytical solutions by engineers and with the advents of computer technology. In several cases, different numerical modelling such as domain or differential approach and boundary or integral approach has been used by the computer technology to simulate the tunnel construction. Excavation of a tunnel has some effects on distribution of insitu stresses in the tunnel neighbourhood, and it is formed as a new stress distribution around the tunnel. The construction of a tunnel through urban areas can lead to ground deformations whereby in several cases damage occurred to the overlaying structures and services.

The New Austrian Tunneling Method (NATM) was developed by Rabcewicz (1964) and Muller (1978) in Austria. The usage of NATM was undertaken in shallow and large tunneling into the rock mass; nowadays, the demand of NATM is for the use into the soil mass with different resistance. The method had been established based on the use of surrounding mass strength to stabilize the tunnel with a noncircular and enlargement of tunnel opening. The use of NATM tunneling method had been known to have cost saving and control the settlement. Using this method, after excavation the soft soil area of the cross-section at the tunnel face, shotcrete as a supporting system is applied on the tunnel walls, constituting a thin and flexible shell.

Several researchers had investigated the effects of heading distance on the NATM tunneling operation. However, limited research had been conducted to analyze the effects of different partial face excavation, in particular the sequence of the excavation, on the surface and subsurface settlement. Therefore, this study had been conducted to define the effect of ground movement due to NATM tunneling work. Hence, a specific excavation model needs to be provided to control surface settlements and minimize the potential impact on the surface structures. Thus, transverse, subsurface and longitudinal settlements need to be assessed from several excavation sequence method that can be conducted using a suitable excavation model.

## **1.2 Problem Statement**

NATM is used in the large sized tunnels; so, full face excavation in the soils may cause large movements around the excavation area and subsequently at the ground surface. To reduce the stress distribution and yield zone around the tunnel, NATM tunneling offered the cross-section which is divided into several suitable parts in area and shape, in order to allow efficient and practical construction. The cross-section is excavated in each stage of excavation sequence. One of the important features in NATM tunneling is excavation sequences design, which depends on soil condition, tunnel geometry and tunneling requirements which may reduce the yield

zone, consequently to minimize the displacement. On the other hand, heading distance may cause the main settlement with respect to various cross-sections.

Karaj Metro Tunnel had been constructed in urban areas with NATM method. The minimum settlement is important to avoid structures damage in the cities. In order to determined the optimum excavation sequence, several cross-sections with different multiface excavation with specific heading distance had been introduced for the Karaj Metro Tunnel. The one that gives the least surface settlement is recommended as the excavation model.

### **1.3 Aim and Objectives of the Study**

The aim of this study is to determine an “excavation model” in NATM Tunneling that cause minimum surface and subsurface settlements using finite element method. The Karaj Metro Tunnel, Iran had been taken as the case study. Hence, the objectives of this research are:

1. To obtain the optimum excavation sequence by analyzing the ground response due to NATM tunneling using two dimensional finite element simulations for various excavation sequences, based on minimum surface settlement.
2. To predict the transverse and longitudinal surface settlements at different face-distance for the recommended excavation model and to compare with the results from existing Karaj Metro Tunnel excavation method, using three dimensional finite element simulations.
3. To determine the effect of different heading distance on the transverse and longitudinal settlements for both the Karaj Metro Tunnel and the recommended excavation models.

#### **1.4 Significance of the Study**

Based on this study the optimum excavation sequence and heading distance in NATM had been recommended. This could minimize the ground displacement and consequently building damages for the tunneling work through the same soil condition at the Karaj Metro Tunnel, Iran. Results could be used by tunneling engineers to plan the tunneling work of large diameters tunnel through soil, to obtain optimum performance.

#### **1.5 Scope and Limitation of the Study**

The research is limited to the immediate settlement occurred during construction using NATM operation through alluvial deposits that contains mainly of sand with appreciable amount of silt (SM-SC) at Line 2 of the Karaj Metro Tunnel, Iran. The long term time dependent consolidation settlement is not covered in this research.

In order to verify and evaluate the accuracy of the finite element (FE) models by the use of Plaxis and Abaqus software, the results had been compared with field measurement during the construction of Karaj Metro Tunnel project in Iran.

## REFERENCES

- Abaqus (2010). *User's manual*. Hibbitt, Karlson and Sorenson, Inc.
- Attewell, P. B. and Woodman, J. P. (1982). Predicting the dynamics of ground settlement and its derivative caused by tunneling in soil, *Ground Engineering*. vol. 15. No. 8, pp. 13-22.
- Bowers, K. H. (1997). *An appraisal of the New Austrian Tunnelling Method in soil and weak rock*. Ph. D. thesis, The University of Leeds.
- Brinkgreve, R. B. J. (2002). Plaxis 2D version 8 Manual. Balkeme Publishers. the Netherlands.
- Budavari, S. (1983). *Rock mechanics in mining practice*, The South African Institute of Mining and Metallurgy.
- Darya Khak Pey Consulting Engineers Inc. (2005). Geotechnical Report of Line 2 of Karaj Metro Tunnel.
- Farias, M.M., Junior, A.H.M., and Assis, A.P. (2004). Displacement Control in Tunnels Excavated by the NATM: 3-D Numerical Simulations, *Tunnelling and Underground Space Technology*. Vol 19, pp. 283–293.
- Gnilsen, R. (1989). Underground Structures Design and Instrumentation, *Elsevier, Amsterdam*, pp. 84-128.
- Helwany, S., and Wiley (2007). *Applied Soil Mechanics with ABAQUS Application*. United State of America: Wiley, J.
- Heuer, R.E. and Virgens, D.L. (1987). Anticipated Behavior of Silty Sands in Tunneling, *Rapid Excavation and Tunneling Conference, New Orleans*, Vol 1, pp. 221-237.
- ICE (1996). Design and Practice Guides. *Sprayed concrete lining (NATM) for tunnels in soft ground*, Institution of Civil Engineers (ICE), London: Telford, T.

- Karakus, M. Fowell, R.J. (2003). Effect of different tunnel faces advance excavation on the settlement by FEM, *Tunnelling and Underground Space Technology*, pp. 513-523.
- Karakus, M., Fowell, R.J. (2004). An Insight into the New Austrian Tunneling Method (NATM), *ROCKMEC'2004-VIIth Regional Rock Mechanics Symposium*, Sivas, Turkey.
- Karakus, M., Fowell, R.J. (2005). Back analysis for tunneling induct ground movement and stress redistribution, *Tunnelling and Underground Space Technology*, 514-524.
- Kolymbas, D. (2005). *A Rational Approach to Tunneling*, Act a Geotechnical Amsterdam, Poland.
- Lee, S.W., Bolton, M.D., Mair, R.J., Hagiwara, T., Soga, K., and Dasari, G.R. (2001). *Centrifuge modelling of injections near tunnel linings*. International Journal of Physical Modelling in Geotechnics, Vol. 1, pp. 9-24.
- Marcher, T., and Jircny, F. (2004). Interaction of primary lining and final lining of a NATM tunnel with respect to relevant longterm effects. *Winter School of Rock Mass Mechanics*, Zakopane, Poland.
- Masin, D. (2009). 3D modelling of an NATM tunnel in High  $K_0$  Clay Using two Different Constitutive Models. *Geotechnical and Geoenvironmental Engineering ASCE*, pp:1326-1335.
- McWilliam, F. (1991). Jet setting under Bonn, *Tunnels & Tunnelling International*, pp. 29–31.
- Megaw, T.M. and Bartlett, V. (1981). *Tunnels: Planning, Design, Construction*. Chichester, West Sussex : Ellis Horwood ; New York : Halsted Press.
- Moller, S.C. Vermeer, P.A. (2008). On numerical simulation of tunnel installation. *Tunnelling and Underground Space Technology*. 23: 461-475.
- Moller, S. (2006) *Tunnel induced settlement and structural forces in lining*, *Ph.D. Thesis*, Univrsitaet Stuttgart.
- Morrison, R.G.K. (1976). *A Philosophy of Ground Control*. Department of Mining and Metallurgical Engineering: Rev. & enl. ed edition.
- Muller, L. (1978). The reasons for unsuccessful applications of the New Austrian Tunnelling Method, *Tunnelling Under Difficult Conditions*, *International Tunnel Symposium*, Tokyo, Pergamum. pp. 67-72.
- Murphy, P. (1993). Design and construction of the A20 Round Hill Tunnels. *Tunnels & Tunnelling International*, pp. 23–25.



- Ng, C.W.W., Lee, K.M., and Tang, D.K.W. (2004). Three-Dimensional Numerical Investigations of New Austrian Tunnelling Method (NATM) Twin Tunnel Interactions. *Canadian Geotechnical Journal*. vol. 41: 523–539.
- Peck (1969). Deep excavation and tunnelling in soft soils. *7<sup>th</sup> International Conference on Soil Mechanics and Foundation Engineering*. Mexico City. pp. 225-290.
- Pound, C., Casson, E.M., Thomas, A.H., and Powell, D.B. (2003). Predicted and observed ground movements around a Sprayed Concrete Lined (SCL) Tunnel in a dense conglomerate. *Underground Construction*.
- Powell, D.B., Field, D.P., and Hulsén (2001). *Rapid Excavation and Tunneling Conference Proceedings*. Design Of An NATM Tunnel For Mission Valley Light Rail East Extension. SME, San Diego.
- Rabcewicz, L. (1965). Die Neue Österreichische Tunnelbauweise, Entstehung, Ausführungen und Erfahrungen, *Der Bauingenieur*, 40. Jg., Heft 8.
- Sahel Consulting Engineers (2005). Geotechnical Consultation of Karaj, Technical Report.
- Sugiyama, T., Hagiwara, T., Nomoto, T., Nomoto, M., Ano, Y., Mair, R.J., Bolton, M.D., and Soga, K. (1999). *Observations of ground movements during tunnel construction by slurry shield method at the docklands light railway Lewisham extension - East London*. Soils and Foundations, Vol. 3, pp. 99-112.
- Tan, T. S., Setiaji, R. R., and Hight, D. W. (2005). Numerical analyses using commercial software– A black box. *Proceedings of Underground Singapore*. pp. 250-258.
- Terzaghi, K. (1950). *Geologic Aspects of Soft Ground Tunneling*. P.D.Trask (ed.), New York: John Wiley and Sons.
- Toan, N.D. (2006). *TBM and Lining Essential Interfaces*, Ph.D. Thesis, Politecnico di Torino, Italy.
- Tunnel Rod (2011). Tunnel Rod Construction Consulting Engineers Inc. Instrumentation Report of Line 2 of Karaj Metro Tunnel, Technical Report.
- Wallis, D. (2000) *Tunnel lining design guide*, Institution of Civil Engineers (ICE), London: Telford T.
- Yoo, C. (2009). Performance of Multi-Faced Tunnelling-A 3D Numerical Investigation. *Tunneling and Underground Space Technology*, Vol. 24, pp. 562-573.